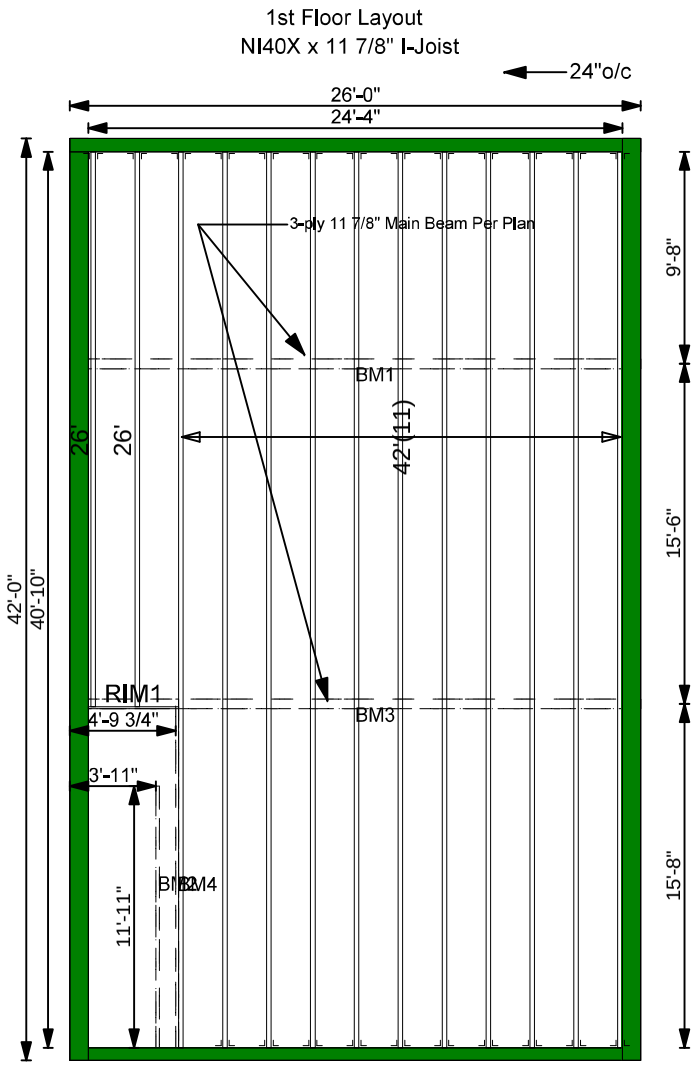
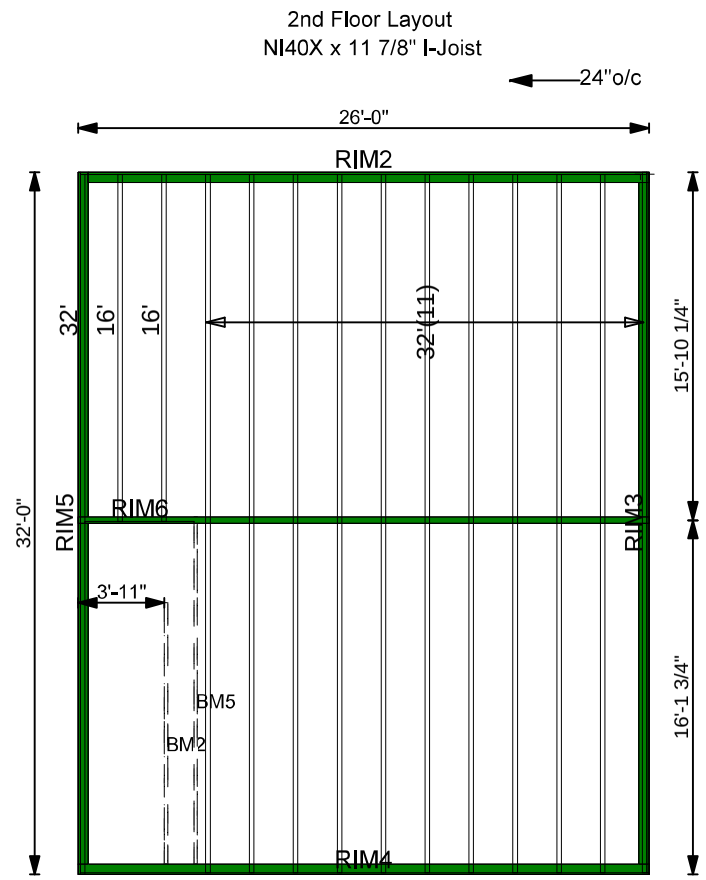




FABCO BUILDING COMPONENTS

Watertown New York

315-782-5283 fax: 315-788-9481

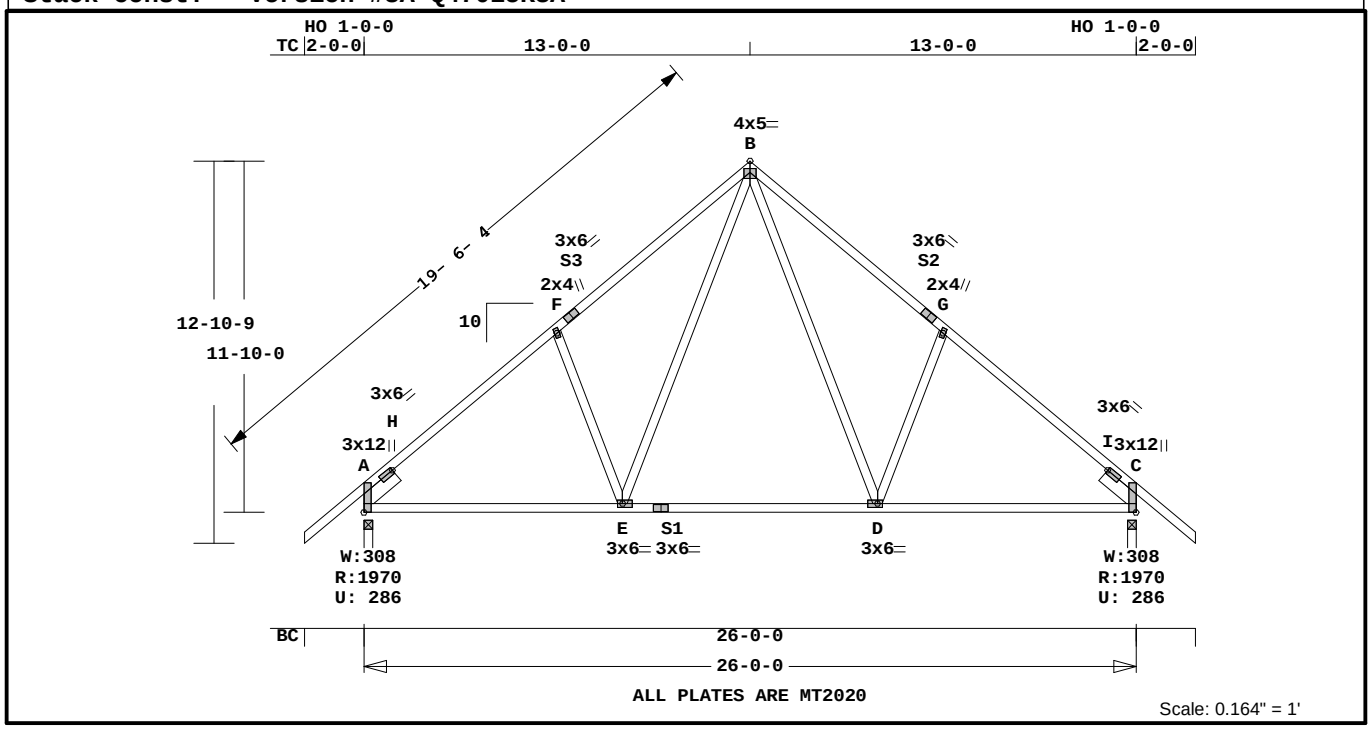
Roof Truss Placement Plan

Floor Loading
TC Live: 40.00 psf
TC Dead: 10.00 psf
BC Live: 0.00 psf
BC Dead: 5.00 psf
TC Stress Inc: 0.00
BC Stress Inc: 0.00
Spacing: 1- 4- 0 o.c.

Account:
Job: N140052FL
Designer:
Checker:
Date: 03-14-14

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
Q47015R3A	A1	15	FI	260000	10	2- 0- 0	2- 0- 0	

Stack Const. - Version #3A Q47015R3A



MiTek® Online Plus™ APPROX. TRUSS WEIGHT: 135.8 LBS

Online Plus -- Version 30.0.023
RUN DATE: 14-MAR-14

	CSI	-Size-	----Lumber----
TC	0.90	2x 4	SPF-1650f-1.5E
BC	0.82	2x 4	SPF-2100f-1.8E
WB	0.58	2x 4	SPF-#2
--	0.44	2x 4	SPF-STUD
	F -E	D -G	
SL	0.13	2x 6	SPF-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	26- 0- 0
or	30.0"	0- 0- 0	26- 0- 0
BC	120.0"	0- 0- 0	26- 0- 0

psf-Ld	Dead	Live
TC	10.0	38.5
BC	10.0	0.0
TC+BC	20.0	38.5

Total 58.5 Spacing 24.0"
Lumber Duration Factor 1.15
Plate Duration Factor 1.15
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	1971	286 U	260 R
C	1971	286 U	260 R

Jt	Brg Size	Required
A	3.5"	2.5"
C	3.5"	2.5"

Plus 6 Wind Load Case(s)
Plus 2 Unbalanced Load Cases
Plus 1 UBC LL Load Case(s)
Plus 1 BC LL Load Case(s)
Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -H	0.80	773 C	0.00 0.80
H -F	0.83	1912 C	0.03 0.80
F -S3	0.58	1866 C	0.02 0.56
S3-B	0.90	1666 C	0.04 0.86
B -S2	0.90	1666 C	0.04 0.86
S2-G	0.58	1866 C	0.02 0.56
G -I	0.83	1912 C	0.03 0.80
I -C	0.80	773 C	0.00 0.80
--------Bottom Chords---------			
A -E	0.82	1449 T	0.13 0.69
E -S1	0.45	1007 T	0.08 0.37
S1-D	0.48	1007 T	0.09 0.39
D -C	0.82	1449 T	0.13 0.69
----------Webs----------			
F -E	0.44	499 C	
E -B	0.58	861 T	
B -D	0.58	861 T	
D -G	0.44	499 C	
----------Sliders----------			
A -H	0.13	1516 C	
I -C	0.13	1516 C	

TL Defl -0.42" in E -D L/729
LL Defl -0.26" in E -D L/999
Shear // Grain in H -H 0.82

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area
Plate - HNGE 18 Ga, Gross Area
Plate - MT8H 18 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	MT20	3.0x12.0	1.5	0.5	0.78
H	MT20	3.0x 6.0	-1.1	-0.9	0.50
F	MT20	2.0x 4.0	Ctr	Ctr	0.38
S3	MT20	3.0x 6.0	Ctr	Ctr	0.61
B	MT20	4.0x 5.0	Ctr	-0.5	0.82
S2	MT20	3.0x 6.0	Ctr	Ctr	0.61
G	MT20	2.0x 4.0	Ctr	Ctr	0.38
I	MT20	3.0x 6.0	1.1	-0.9	0.50
C	MT20	3.0x12.0	-1.5	0.5	0.78
E	MT20	3.0x 6.0	1.0	Ctr	0.96
S1	MT20	3.0x 6.0	Ctr	Ctr	0.39
D	MT20	3.0x 6.0	-1.0	Ctr	0.96

REFER TO ONLINE PLUS GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
FABCO BUILDING COMPONENTS
Analysis Conforms To:
IBC/IRC2006
TPI 2002
OH Loading
Soffit psf 2.0
This truss has been designed for 20.0 psf LL on the B.C. in areas where a rectangle 3- 6- 0 tall by 2- 0- 0 wide will fit between the B.C. and any other member.
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-05
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 90 mph
Mean Roof Height: 25-0
Exposure Category: C
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 6.0 psf
BC Dead Load : 6.0 psf
Roof Snow Load per ASCE 7-05
Ground Snow Load = 50.0 psf
Rain Load = 0.0 psf
Importance Factor 1.00
Thermal Factor 1.10
Exposure Factor 1.00
Balanced Load Factor 0.72
Unbal snow loads checked
Par-to-ridge not req'd
Max comp. force 1912 Lbs
Max tens. force 1449 Lbs
Connector Plate Fabrication Tolerance = 20%
This truss is designed for a creep factor of 1.5 which is used to calculate total load deflection.

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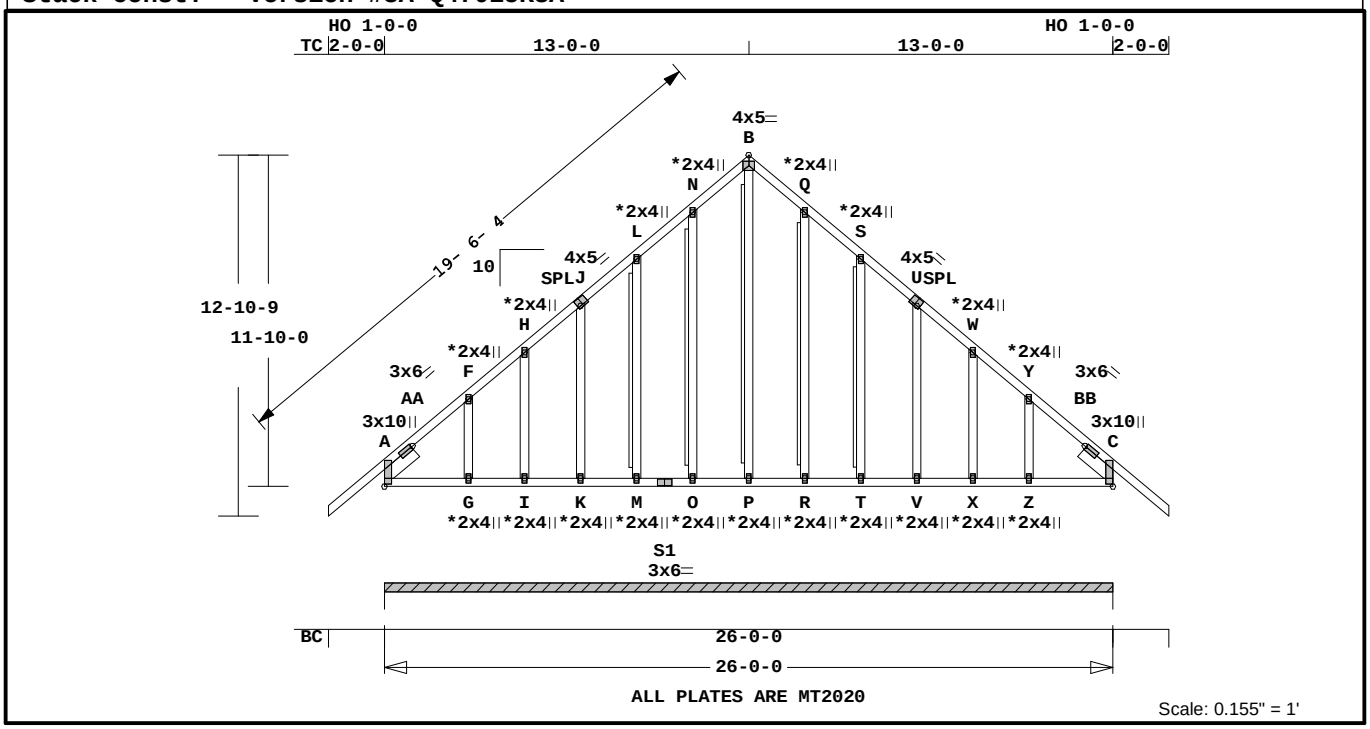
Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
Q47015R3A	*A1E*	2	FI	260000	10	2- 0- 0	2- 0- 0	

Stack Const. - Version #3A Q47015R3A



MiTek® Online Plus™ APPROX. TRUSS WEIGHT: 183.9 LBS

Online Plus -- Version 30.0.023
RUN DATE: 14-MAR-14

	CSI	-Size-	----Lumber----
TC	0.08	2x 4	SPF-1650f-1.5E
BC	0.08	2x 4	SPF-2100f-1.8E
GW	0.17	2x 4	SPF-STUD
--	0.17	2x 4	SPF-#2
	M -L	O -N	P -B R -Q
	T -S		
SL	0.01	2x 6	SPF-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	26- 0- 0
or	48.0"	0- 0- 0	26- 0- 0
BC	120.0"	0- 0- 0	26- 0- 0

One 2x3 T-Brace
M -L O -N R -Q T -S
One 2x4 T-Brace
P -B
Attach to 1-1/2" edge w/10d nails at 6" o.c. T-Brace must cover 90% of web length and have a MOE >= 1.40E6.

psf-Ld	Dead	Live
TC	10.0	38.5
BC	10.0	0.0
TC+BC	20.0	38.5

Total 58.5 Spacing 24.0"
Lumber Duration Factor 1.15
Plate Duration Factor 1.15
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	3446	573 U	260 R

Jt	Brg Size	Required
A	312.0"	0"-to- 312"

Plus 6 Wind Load Case(s)
Plus 2 Unbalanced Load Cases
Plus 1 UBC LL Load Case(s)
Plus 1 DL Load Case(s)

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -AA	0.05	98 C	0.00 0.05
AA-F	0.07	180 C	0.00 0.07
F -H	0.07	134 C	0.00 0.07
H -J	0.05	116 C	0.00 0.05
J -L	0.07	190 T	0.00 0.07
L -N	0.08	290 T	0.00 0.08
N -B	0.08	367 T	0.00 0.08
B -Q	0.08	367 T	0.00 0.08
Q -S	0.08	290 T	0.00 0.08
S -U	0.07	190 T	0.00 0.07
U -W	0.05	116 C	0.00 0.05
W -Y	0.07	134 C	0.00 0.07
Y -BB	0.07	180 C	0.00 0.07
BB-C	0.05	98 C	0.00 0.05
--------Bottom Chords---------			
A -G	0.08	3 T	0.00 0.08
G -I	0.02	0 T	0.00 0.02
I -K	0.01	0 T	0.00 0.01
K -M	0.01	0 T	0.00 0.01
M -S1	0.01	0 T	0.00 0.01
S1-O	0.01	0 T	0.00 0.01
O -P	0.01	0 T	0.00 0.01
P -R	0.01	0 T	0.00 0.01
R -T	0.01	0 T	0.00 0.01
T -V	0.01	0 T	0.00 0.01
V -X	0.01	0 T	0.00 0.01
X -Z	0.02	0 T	0.00 0.02
Z -C	0.08	3 T	0.00 0.08
----------Gable Webs----------			
G -F	0.06	223 C	
I -H	0.09	186 C	
K -J	0.17	194 C	
M -L	0.08	251 C	1T-Br
O -N	0.13	307 C	1T-Br
P -B	0.17	392 C	1T-Br
R -Q	0.13	307 C	1T-Br
T -S	0.08	251 C	1T-Br
V -U	0.17	194 C	
X -W	0.09	186 C	
Z -Y	0.06	223 C	
----------Sliders----------			
A -AA	0.01	190 C	
BB-C	0.01	190 C	

TL Defl 0.00" in A -G L/999
LL Defl 0.00" in A -G L/999
Shear // Grain in N -B 0.22

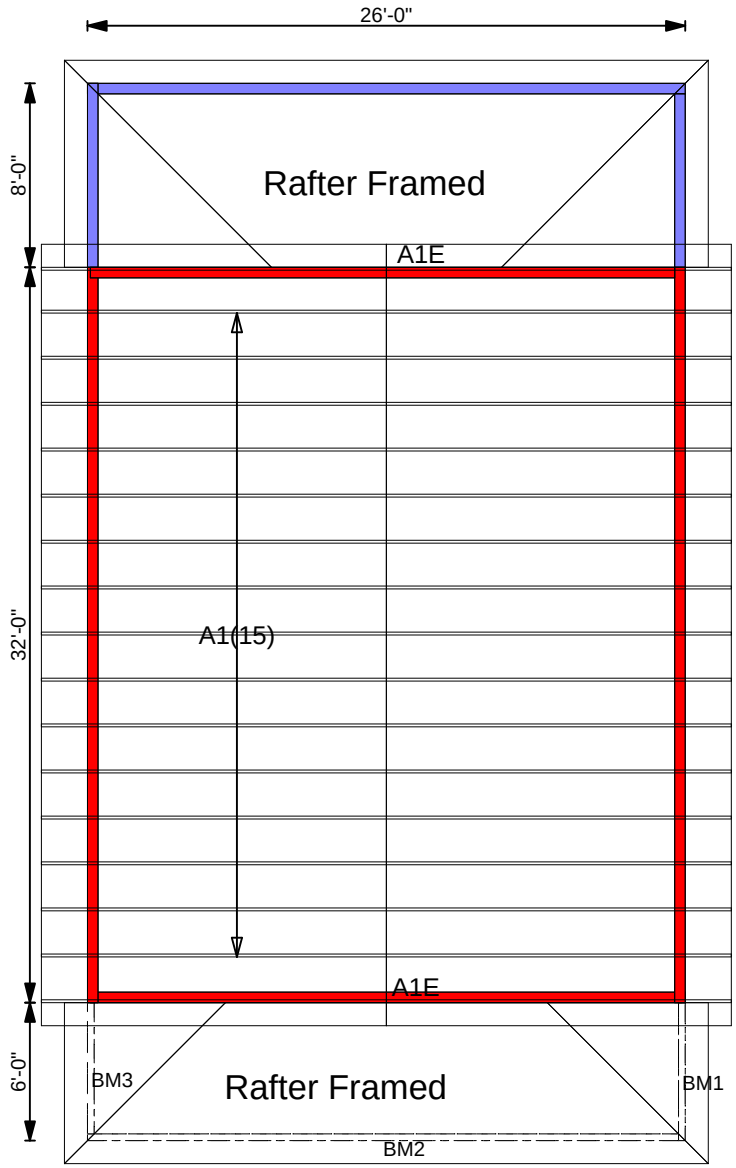
Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area
Plate - HNGE 18 Ga, Gross Area
Plate - MT8H 18 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	MT20	3.0x10.0	1.5	0.7	0.49
AA	MT20	3.0x 6.0	-1.1	-0.9	0.19
F	MT20	2.0x 4.0	Ctr	Ctr	0.00
H	MT20	2.0x 4.0	Ctr	Ctr	0.00
J	MT20	4.0x 5.0	-0.7	0.9	0.42
L	MT20	2.0x 4.0	Ctr	Ctr	0.00
N	MT20	2.0x 4.0	Ctr	Ctr	0.00
B	MT20	4.0x 5.0	Ctr	Ctr	0.37
Q	MT20	2.0x 4.0	Ctr	Ctr	0.00
S	MT20	2.0x 4.0	Ctr	Ctr	0.00
U	MT20	4.0x 5.0	0.7	0.9	0.42
W	MT20	2.0x 4.0	Ctr	Ctr	0.00
Y	MT20	2.0x 4.0	Ctr	Ctr	0.00
BB	MT20	3.0x 6.0	1.1	-0.9	0.19
C	MT20	3.0x10.0	-1.5	0.7	0.49
G	MT20	2.0x 4.0	Ctr	Ctr	0.00
I	MT20	2.0x 4.0	Ctr	Ctr	0.00
K	MT20	2.0x 4.0	Ctr	Ctr	0.00
M	MT20	2.0x 4.0	Ctr	Ctr	0.00
S1	MT20	3.0x 6.0	Ctr	Ctr	0.39
O	MT20	2.0x 4.0	Ctr	Ctr	0.00
P	MT20	2.0x 4.0	Ctr	Ctr	0.00
R	MT20	2.0x 4.0	Ctr	Ctr	0.00
T	MT20	2.0x 4.0	Ctr	Ctr	0.00
V	MT20	2.0x 4.0	Ctr	Ctr	0.00
X	MT20	2.0x 4.0	Ctr	Ctr	0.00
Z	MT20	2.0x 4.0	Ctr	Ctr	0.00

REFER TO ONLINE PLUS GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
FABCO BUILDING COMPONENTS
Analysis Conforms To:
IBC/IRC2006
TPI 2002
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified Building Designer as per ANSI/TPI 1.
Wind Loads - ANSI / ASCE 7-05
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 90 mph
Mean Roof Height: 25-0
Exposure Category: C
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 6.0 psf
BC Dead Load : 6.0 psf
Roof Snow Load per ASCE 7-05
Ground Snow Load = 50.0 psf
Rain Load = 0.0 psf
Importance Factor 1.00
Thermal Factor 1.10
Exposure Factor 1.00
Balanced Load Factor 0.72
Unbal snow loads checked
Par-to-ridge not req'd
Max comp. force 392 Lbs
Max tens. force 367 Lbs
Connector Plate Fabrication Tolerance = 20%
This truss is designed for a creep factor of 1.5 which is used to calculate total load deflection.

Option#3 - Common Trusses Spaced @ 24"o/c



	FABCO BUILDING COMPONENTS Watertown New York 315-782-5283 fax: 315-788-9481 Roof Truss Placement Plan	Roof Loading TC Live: 38.50 psf TC Dead: 10.00 psf BC Live: 0.00 psf BC Dead: 10.00 psf TC Stress Inc: 15.00 BC Stress Inc: 15.00 Spacing: 2- 0- 0 o.c.	Account: Job: N140052TR3A Designer: Checker: Date: 03-14-14